

# Project Report on Analysis of steel fibre concrete with varying Aggregate grading

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## CERTIFICATE

This is to certify that the following students have satisfactorily carried out B.E. project work entitled "Analysis of steel fibre concrete with varying Aggregate grading"

This work is being submitted for the award of degree of Bachelor of Civil Engineering. It is submitted in the partial fulfilment of the prescribed syllabus of Savitribai Phule Pune University, Pune for the academic year 2020-2021.

## ACKNOWLEDGEMENT

We hereby are grateful to be able to present our project on the topic "Analysis of steel fibre concrete with varying Aggregate grading: And we would like to give thanks to the people who have helped and supported us through this.

We wish to express our deepest thanks to our project guide and lecturer Asst. Prof. Amol More sir for helping us with his valuable advice and ever ready support during making of this project. Without his counsel, keen insight and positive reinforcement, we wouldn't have been able to deliver our very best.

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We would also like to thank our institution and faculty members without whom the seminar would have been a distant reality. We also extend thanks to our family members and our well-wishers who have stood by us.

## ABSTRACT

The usefulness of fiber reinforced concrete (FRC) in various civil engineering applications is indisputable. Fiber reinforced concrete has so far been successfully used in slabs on grade, architectural

panels, precast products, offshore structures, structures in seismic regions, thin and thick repairs, crash barriers, footings, hydraulic structures and many other applications. Fiber Reinforced Concrete (FRC) is gaining attention as an effective way to improve the performance of concrete. Fibers are currently being specified in tunneling, bridge decks, pavements, loading docks, thin unbonded overlays, concrete pads, and concrete slabs. These applications of fiber reinforced concrete are becoming increasingly popular and are exhibiting excellent performance.

Fiber-reinforced concrete (FRC) is concrete containing fibrous material which increases its structural integrity. It contains short discrete fibers that are uniformly distributed and randomly oriented. Fibers include steel fibers, glass fibers, synthetic fibers and natural fibers. This study presents understanding of strength of fiber reinforced concrete. Mechanical properties and durability of fiber reinforced concrete.

## A) Relevance

Compared to other building materials such as metals and polymers, concrete is significantly more brittle and exhibits a poor tensile strength. Based on fracture toughness values, steel is at least 100 times more resistant to crack growth than concrete. Concrete in service thus cracks easily and this cracking creates easy access routes for deleterious agents resulting in early saturation, freeze-thaw damage, scaling, discoloration and steel corrosion.

The concerns with the inferior fracture toughness of concrete are alleviated to a large extent by reinforcing it with fibers of various materials. The resulting material with a random

distribution of short, discontinuous fibers is termed fiber reinforced concrete (FRC) and is slowly becoming a well accepted mainstream construction material. Significant progress has been made in the last thirty years towards understanding the short and long-term performances of fiber reinforced cementitious materials, and this has resulted in a number of novel and innovative applications.

Concrete is one of the most versatile building materials. It can be cast to fit any structural shape from a cylindrical water storage tank to a rectangular beam or column in a high rise building. The advantages of using concrete include high compressive strength, good fire resistance, high water resistance, low maintenance, and long service life.

The disadvantages of using concrete include poor tensile strength, low strain at fracture and formwork requirement. The major disadvantage is that concrete develops microcracks during curing. It is the rapid propagation of these micro cracks under applied stress that is responsible for the low tensile strength of the material. Hence fibers are added to concrete to overcome these disadvantages.

The addition of fibers in the matrix has many important effects. Most notable among the improved mechanical characteristics of Fiber Reinforced Concrete (FRC) are its superior fracture strength, toughness, impact resistance, flexural strength resistance to fatigue, improving fatigue performance is one of the primary reasons for the extensive use of Steel Fibre Reinforced Concrete (SFRC) in pavements, bridge decks, offshore structures and machine foundation, where the composite is subjected to cyclically varying load during its lifetime.

The main reasons for adding steel fibres to concrete matrix is to improve the post-cracking response of the concrete, i.e., to improve its energy absorption capacity and apparent ductility and to provide crack resistance and crack control. Also, it helps to maintain structural integrity and cohesiveness in the material. The initial researches combined with the large volume of follow up research have led to the development of a wide variety of material formulations that fit the definition of Fibre Reinforced Concrete.

## B) Present Theories and Practices

Numerous studies have been carried out to study the Analysis of steel fibre concrete with varying Aggregate grading. A few of the studies are reviewed and briefly mentioned as follows.

Haider M. Al-Baghdadi, Faiz H. Al-Merib (2021) Department of Civil Engineering, College of Engineering, University of Babylon, Babylon, Iraq. This study is to investigate the influence of steel and synthetic fiber parameters, along with different coarse aggregate maximum sizes (CAMZs) on FRC performance. Additionally, in past research, the empirical relationships among the compressive, tensile, and flexural strengths of plain concrete and FRC were assessed, and correlations between these mechanical properties of FRC were examined. For each CAMZ, four fiber dosages for each fiber type were considered.

M. Acikgens (2015) studied carried out the study Based on our experimental results on the effects of gradation and  $D_{max}$  on SFRC properties with constant cement dosages and W/C ratios, the following conclusions can be drawn.

In addition to reduce of the workability by using 1% steel fibres, most of the slump values of the SFRC mixture were 0. The results of the Ve-Be tests were more variable than the slump tests for SFRC. Changing the aggregate grading had a noticeable effect on the workability of both SFRC and reference concrete. The finest and the coarsest grading showed low workability for both  $D_{max}$  values. SFRC mixtures with smaller  $D_{max}$  were more workable.

Dr. D.A. Sinha (2017) Addition of 1% steel fibres result in higher compressive strength and use of more than 1% steel fibres will bring down the compressive strength. Addition of 1% steel fibres result in higher tensile strength and use of more than 1% steel fibres will bring down the tensile strength

Flexural strength is found to increase as the percentage of steel fibres in it increases

Prof. Kalpan. Sutar (2015) The addition of fibers in concrete specimens like Cubes and Prisms here is a increase in strength up to some percentage level. The maximum percentage increase in compressive strength at 75% fibre content it was 9.2%. The corresponding increase in flexural and strength value were 21.21% for 28 days.

Archana Dongre (2017) A brief state-of-the-art report on fiber reinforced concrete is presented. Our understanding of fiber-matrix interaction, reinforcement mechanisms and performance characteristics is fairly advanced.

Fiber reinforced concrete is a promising material to be used in the Middle-East for sustainable and long-lasting concrete structures. Its performance has already been proven in other hot and arid climates and in other chemically deleterious environments. Fiber reinforced concrete pavements prove to be more efficient than conventional RC pavements, in several aspects.

K. Srinivasa Rao (2013) studied An increase in compressive strength and tensile strength has been observed for both standard concrete and fibre reinforced standard concrete when exposed to temperature of 50°C

Age of concrete has a role in attaining durable concrete, both M30 controlled concrete and M30 steel fibre reinforced concrete suffers more weight loss at later ages compared to early ages of concrete

At 200°C temperature, steel fiber reinforced standard concrete shows a decrease of 60% in weight loss compared to M30 controlled concrete at the age

Ulaka DC (2019) This study shows that concretes of the same mix ratio, maximum size of aggregate and water content will have their strength and workability properties differ if they are subject to a change in aggregate gradation. It shows that as the fineness modulus increases, the concrete becomes weaker but more workable. Finally the density of concrete is not affected by the maximum aggregate size or the grading of aggregates in the mix and could be the reason why the unit weight of mass concrete is usually specified as 24 kN/m<sup>3</sup> irrespective of the concrete mix ratio.

Prasad Rangaraju (2013) whether coarse aggregates or fine aggregates, failing to meet the standard SCDOT specifications have a broad range of impacts on various properties of concrete, depending on a number of factors. The impact on concrete properties ranges from nothing significant on certain properties (such as compressive strength, modulus of elasticity, density and others) to significant on certain other selected properties such as split tensile strength and rapid chloride ion permeability among others. The specific impact of a failed aggregate gradation not only depends on whether the aggregates fail on the coarser or the finer side of the gradation but also on the extent of the failure away from the acceptable gradation limits.

### C) Scope of work

The aim of this study is to investigate the impact of the steel and synthetic fiber parameters (fiber length, diameter, and shape), a

long with the CAMZ, on the workability, compressive, tensile, and flexural strengths of FRC. The fiber length ( $l_f$ ) from 13 mm to 60 mm, CAMZ from 9.5 mm to 37.5 mm, and ratio of  $l_f$ /CAMZ in the range of 0.35–5.68 were conducted in order for the results to be used in the proposed logical range of the  $l_f$ /CAMZ ratio. For each CAMZ, four fiber dosages of 0.0%, 0.5%, 1.0%, and 1.5% by the volume of concrete for each fiber type were considered. Moreover, in this research, correlations among the flexural, splitting tensile, and compressive strengths of synthetic/steel FRC with different fiber parameters were analyzed and evaluated with past research empirical relations.

### D) Proposed work

The objectives of proposed work are listed below:

1. The hardened properties, such as the compressive, splitting tensile and flexural strengths, were also analyzed
2. This study investigates the effects of changing the aggregate grading and maximum aggregate
3. Analysis of different fiber types
4. In addition, the toughness of the SFRC was calculated
- 5.

### E) Expected Date of completion: June 2021

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#### NOTATIONS

1. FRC–Fibre Reinforced Concrete
2. CAMZ– Coarse Aggregate Maximum Size
3. Lf–Length of Fibre
4. ASTM–American society for testing Material
5. W/C– Water cement ratio
6. MSF–Micro Steel Fibre
7. HSF–Hooked end Steel Fibre
8. MSYF– Micro Synthetic Fibre
9. G10–10 mm Coarse Aggregate
10. G19 – 19 mm Coarse Aggregate

### I. INTRODUCTION

#### 1.1 Introduction of the Project Work

Concrete is identified to be weak in resisting tensile stresses and can easily crack under low-

level tensile forces. Incorporating fiber into the concrete mixture is a typical method to modify concrete material. Due to the distribution of fiber in the concrete mixture, the mechanical properties of fiber reinforced concrete (FRC) might be improved. The amount of enhancement of the performance of a concrete structure is mainly proportional to the volume fraction, aspect ratio, fiber geometry, fiber distribution, and fiber orientation.

One of the effects on the fiber orientation and distribution in the concrete matrix is the coarse aggregate maximum size (CAMZ), which significantly impacts the mechanical properties of FRC. With the CAMZ increased from 3 mm to 14 mm, the tensile strength and elasticity modulus reduce, while the fracture energy of concrete specimens is enhanced. Appa Rao and Raghu Prasad found that the fracture toughness and fracture energy of concrete were enhanced due to the increase of the CAMZ from 4.75 mm to 20 mm.



Fig.1 Fiber Reinforced Concrete

Past research has investigated the effect of CAMZs of 8, 13, and 20 mm and fiber dosages of 0.0%, 1.0%, and 2.0% on the flexural performance of steel FRC. According to Olivito and Zuccarelli, to ensure a uniform and efficient fiber distribution, the steel fiber length should be two times more than the CAMZ. Additionally, steel FRC with a small CAMZ shows better flexural behavior, and the CAMZ of steel FRC should not go beyond a three-quarter steel fiber length [16]. The ratio of the length of the steel fiber to the CAMZ influences the mechanical properties of

steel FRC as much as the fiber content

The steel fiber parameters have substantial effects on the properties of FRC. FRC with a 60 mm length of steel fiber exhibits higher flexural strength and fracture strength than the concrete with a 30 mm steel fiber length. On the other hand, Doo-Yeol Yoo et al. reported that the incorporation of a 30 mm fiber length in concrete showed less improvement in flexural performance compared with the FRC with a 13–19.5 mm fiber length. Other fiber parameters that impact the FRC properties are the fiber type and fiber shape, which



may influence the dynamic and static concrete properties. The distribution and orientation of steel fiber have considerably influenced concrete performance. According to the investigations of Mert Yücel Yardimci et al. and Lee and Kim, the CAMZ and properties of steel fiber influence the fiber fracture energy and orientation of steel FRC. In addition, another study by Su Tae Kang et al. reported that the ultimate flexural strength of steel FRC was mainly affected by the fiber distribution characteristics, with less impact on the first cracking strength.

Research to date on using steel fiber in concrete mixtures indicates an improvement in the mechanical properties of FRC. However, there is no study examining the effect of the CAMZ on the behavior of synthetic FRC. Additionally, for steel FRC, past research has investigated the impact of CAMZs less than 25 mm on concrete performance, along with limited studies on CAMZs larger than 25 mm. Therefore, in order to find optimal fiber strengthening and toughening with different CAMZs, it is necessary to understand the correlations among the flexural, splitting tensile, and compressive strengths of synthetic/steel FRC with different fiber parameters. Studies about the relationship between fiber parameters and CAMZs of FRC are limited.

## 1.2 Problem Statement

Plain, unreinforced concrete is a brittle material, with a low tensile strength and a low strain capacity. The role of randomly distributed discontinuous fibers is to bridge across the cracks that develop and provide some post-cracking "ductility". If the fibers are sufficiently strong, sufficiently bonded to material, and permit the FRC to carry significant stresses over a relatively large strain capacity in the post-cracking stage. But as the steel fibers affect the mechanical properties of the concrete positively, they also affect workability negatively.

So need to add some different aggregate grading to achieve good workability with good strength and with optimum water-cement ratio

## 1.3 Objectives

This project was carried out with the following purposes:

12. The hardened properties, such as the compressive, splitting tensile and flexural strengths, were also analyzed
13. This study investigates the effects of changing the aggregate grading and maximum aggregate

14. Analysis of different fiber types

15. In addition, the toughness of the SFRC was calculated

## 1.4 Scope of the Project Work

The aim of this study is to investigate the impact of the steel and synthetic fiber parameters (fiber length, diameter, and shape), along with the CAMZ, on the workability, compressive, tensile, and flexural strengths of FRC. The fiber length ( $l_f$ ) from 13 mm to 60 mm, CAMZ from 9.5 mm to 37.5 mm, and ratio of  $l_f$ /CAMZ in the range of 0.35–5.68 were conducted in order for the results to be used in the proposed logical range of the  $l_f$ /CAMZ ratio. For each CAMZ, four fiber dosages of 0.0%, 0.5%, 1.0%, and 1.5% by the volume of concrete for each fiber type were considered. Moreover, in this research, correlations among the flexural, splitting tensile, and compressive strengths of synthetic/steel FRC with different fiber parameters were analyzed and evaluated with past research empirical relations.

## II. LITERATURE REVIEW

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Dmax values. SFRC mixtures with smaller Dmax were more workable.

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Jeetendra Prajapati (2019)

study of gradation, it can be concluded that the most of the aggregate available are of nominal maximum size 40mm with partially out of gradation limit given by IS383-1970. From mechanical test of aggregate, most of the aggregate samples can be concluded as medium strength aggregate although there is some variation in mechanical strength. The effect of the coarse aggregate source on compressive strength of various nominal mix concrete can be concluded based on the 7 days and 28 days compressive strength test results. The results of 7 and 28 days compressive strength test showed there is significant effect of coarse aggregate sources on the compressive strength of various nominal mix design concrete. As per the research result, keeping other parameters same, variation in coarse aggregate source only can cause up to 47% variation in the 28 days compressive strength.

Archana Dongre(2017)A brief state-of-the-art report on fiber reinforced concrete is presented.Our understanding of fiber-matrix interaction, reinforcement mechanisms and performance characteristics is fairly advanced. Fiber reinforced concrete is a promising material to be used in the Middle-East for sustainable and long-lasting concrete structures. Its performance has already been proven in other hot and arid climates and in other chemically deleterious environments.Fiber reinforced concrete pavements prove to be more efficient than conventional RC pavements, in several aspects

Compressive strength for fibre reinforced concrete is seen to be improved. It can be clearly seen that strength at 28 days for CSFRC 1% is better than other cases hence recommended

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Karthik Obla(2007)Based on the results of this study, it can be concluded that there is no assurance that a concrete specification that includes a requirement for WG through compliance with CF and/or 8-18 charts will lead to reduced mixing water content or lower shrinkage as is typically the goal with these controls on aggregate grading.

The above conclusion does not mean that aggregate grading is unimportant for concrete performance. For example if adequate fine material is not present then the concrete can become prone to segregation, and high bleeding. On the other

hand too much of fine material may make it sticky and difficult to finish.

### III. METHODOLOGY

#### 3.1 Analysis of Fibre Reinforced Concrete

Compared to conventional concrete, fiber reinforced concrete mixes are generally characterized by higher cement factor, higher fine aggregate content and smaller size coarse aggregate. A fiber mix generally requires more vibration to consolidate the mix. External vibration is preferable to prevent fiber segregation. Metal trowels, tube floats, and rotating power floats can be used to finish the surface. Mechanical Properties of FRC Addition of fibers to concrete influences its mechanical properties which significantly depend on the type and percentage of fiber. Fibers with end anchorage and Properties and Applications of Fiber Reinforced Concrete. High aspect ratio were found to have improved effectiveness. It was shown that for the same length and diameter, crimped-end fibers can achieve the same properties as straight fibers using 40 percent less fibers [S]. In determining the mechanical properties of FRC, the same equipment and procedure as used for conventional concrete can also be used. Below are cited some properties of FRC determined by different researchers.

##### Compressive Strength:

The presence of fibers may alter the failure mode of cylinders, but the fiber effect will be minor on the improvement of compressive strength values (0 to 15 percent)

##### Modulus of Elasticity:

Modulus of elasticity of FRC increases slightly with an increase in the fibers content. It was found that for each 1 percent increase in fiber content by volume there is an increase of 3 percent in the modulus of elasticity.

##### Flexure:

The flexural strength was reported to be increased by 2.5 times using 4 percent fibers.

**Toughness:** For FRC, toughness is about 10 to 40 times that of plain concrete.

##### Splitting Tensile Strength:

The presence of 3 percent fiber by volume was reported to increase the splitting tensile strength of mortar about 2.5 times that of the unreinforced one.

##### Fatigue Strength:

The addition of fibers increases fatigue strength of

about 90 percent and 70 percent of the static strength at  $2 \times 10^6$  cycles for non-reverse and full reversal of loading, respectively.

##### Impact Resistance:

The impact strength for fibrous concrete is generally 5 to 10 times that of plain concrete depending on the volume of fiber.

##### Corrosion of Steel Fibers:

A 1 year exposure of steel fibrous mortar to outdoor weathering in an industrial atmosphere showed no adverse effect on the strength properties. Corrosion was found to be confined only to fibers actually exposed on the surface. Steel fibrous mortar continuously immersed in seawater for 10 years exhibited a 15 percent loss compared to 40 percent strength decrease of plain mortar.

##### Structural Behavior of FRC

Fibers combined with reinforcing bars in structural members will be widely used in the future. The following are some of the structural behavior

##### Flexure

The use of fibers in reinforced concrete flexure members increases ductility, tensile strength, moment capacity, and stiffness. The fibers improve crack control and preserve post cracking structural integrity of members.

##### Torsion:

The use of fibers eliminates the sudden failure characteristic of plain concrete beams. It increases stiffness, torsional strength, ductility, rotational capacity, and the number of cracks with less crack width.

##### Shear:

Addition of fibers increases shear capacity of reinforced concrete beams up to 100 percent. Addition of randomly distributed fibers increases shear-friction strength, the first crack strength, and ultimate strength.

##### Column:

The increase of fiber content slightly increases the ductility of axially loaded specimen. The use of fibers helps in reducing the explosive type failure for columns.

##### High Strength Concrete:

Fibers increase the ductility of high strength concrete. The use of high strength concrete and steel produces slender members. Fiber addition will

help in controlling cracks and deflections.

### Cracking and Deflection:

Tests have shown that fiber reinforcement effectively controls cracking and deflection, in addition to strength improvement. In conventionally reinforced concrete beams, fiber addition increases stiffness, and reduces deflection.

## 3.2 Types of Fibres

### 3.2.1 Steel Fiber Reinforced Concrete:

Steel fiber-reinforced concrete is basically a cheaper and easier to use form of rebar reinforced concrete. Rebar reinforced concrete uses steel bars that are laid within the liquid cement, which requires a great deal of prep work but make for a much stronger concrete. Steel fiber-reinforced concrete uses thin steel wires mixed in with the cement. This imparts the concrete with greater structural strength, reduces cracking and helps protect against extreme cold. Steel fiber is often used in conjunction with rebar or one of the other.



FIG3.1 STEEL FIBERS

### 3.1.2 GLASS REINFORCED CONCRETE:

Glass fiber-reinforced concrete uses fiberglass, much like you would find in fiberglass insulation, to reinforce the concrete. The glass fiber helps insulate the concrete in addition to making it stronger. Glass fiber also helps prevent the concrete from cracking over time due to mechanical or thermal stress. In addition, the glass fiber does not interfere with radio signals like the steel fiber reinforcement does.



FIG3.2 GLASS FIBRES



- Very high tensile strength 1020 to 4080 N/mm<sup>2</sup>.
- Shows comparable improvement in durability to conventional E-glass fiber.

### 3.1.3

#### **SYNTHETIC REINFORCED CONCRETE:**

Synthetic fiber-reinforced concrete uses plastic and nylon fibers to improve the

concrete's strength. In addition, the synthetic fibers have a number of benefits over the other fibers. While they are not as strong as steel, they do help improve the cement pumpability by keeping it from sticking in the pipes. The synthetic fibers do not expand in heat or contract in the cold which helps prevent cracking. Finally synthetic fibers help keep the concrete from spalling during impacts or fires.



**FIG 3.3 SYNTHETIC FIBRES**

### 3.1.4

#### **NATURAL FIBER REINFORCED CONCRETE:**

Historically, fiber-reinforced concrete have used natural fibers, such as straw or hair. While these fibers help the concrete's strength they can also make it weaker if too much is used.

In addition if the natural fibers are rotting when they are mixed in then the rot can continue while in the concrete. This eventually leads to the concrete crumbling from the inside, which is why natural fibers are no longer used in construction.



**Fig. 3.4 Strew fibre**

### 3.1.5 ASBESTOS FIBER REINFORCED CONCRETE:

- ✓ Mineral fiber, most successful of all as it can be mixed with portland cement.
- ✓ Tensile strength of asbestos varies between 560 to 980 N/mm<sup>2</sup>.

- ✓ Asbestos cement paste has considerably higher flexural strength than Portland cement paste.
- ✓ For unimportant concrete work, organic fibers like coir, jute and canes splits are also used.



FIG3.5 ASBESTOS FIBRES

### 3.1.6 CARBON FIBER REINFORCED CONCRETE:

- ✓ Possesveryhigh tensilestrength2110 to2815N/mm<sup>2</sup> andYoung'smodulus.
- ✓ Cementcompositeconsistingofcarbonfibersshowveryhighmodulusofelasticityandflexuralstrength.

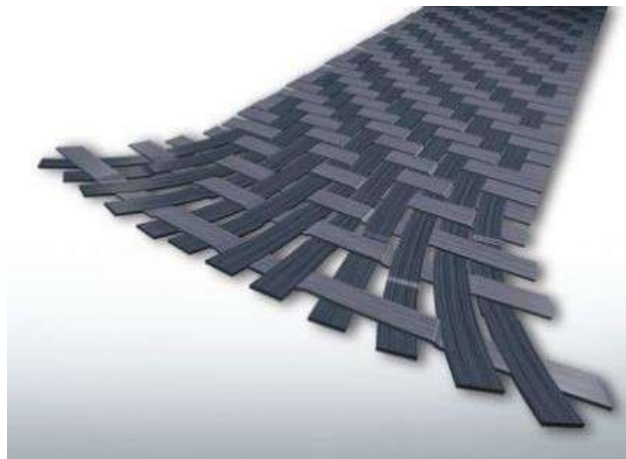


FIG3.6 CARBON FIBRES

## IV. ANALYSE EXPERIMENT PROGRAM BASED ON PAPER STUDY

### 4.1 Material:

The composition of Portland cement utilized in this study is listed in Table-1, in accordance with ASTM .In Table, the physical composition of the cement is presented along with the ASTM limits. River sand (fine aggregate) passed through a 4.75 mm sieve was used. The sieve analysis of the fine aggregate and the passing of the overall limit of ASTM are listed in Table 3 .For the coarse aggregate, three CAMZs were used (CAMZ=9.5mm, 19mm, and 37.5mm), namely G10, G19, and G38, respectively. The sieve analysis of the

coarse aggregates is listed in Table 4 . This table shows that the grading of the coarse aggregate was within the ASTM limits.

Three fiber configurations, namely micro steel fiber, hooked end steel fiber, and macrosynthetic fiber, were used. For the micro steel fiber (HAREX copper-plated micro-filament steel fiber), the fiber length was 13 mm. In addition, for the hooked end steel fiber (HAREX fiber), lengths of 35 mm and 60 mm were used. Fiber lengths of 19, 38, and 54 mm were used for the macrosynthetic fiber (Forta Ferro concrete fiber). The properties of the studied fibers are listed in Table 5 , and Figure shows a photo of each fiber type.

**Table1.Portlandcementcomposition**

Constituent	ChemicalComposition	Cement(TypeI)%byWeight
Aluminumoxide	Al <sub>2</sub> O <sub>3</sub>	5.45
Iron oxide	Fe <sub>2</sub> o <sub>3</sub>	3.41
Magnesia	MgO	3.7
Sulfate	SO <sub>3</sub>	2.25
Tricalciumaluminates	C <sub>3</sub> A	9.85
Tricalciumsilicate	C <sub>3</sub> S	40.43
Diacalciumsilicate	C <sub>2</sub> S	28.1
Tricalciumaluminaferrite	C <sub>4</sub> AF	8.12

**Table2.Physicalcompositionofthecement**

Physical properties	Test results
Fitness, specification (m <sup>2</sup> /kg)	
Turbidimeter test	190
Air permeability	310
Soundness using autoclave method	0.12%
Setting time at which vicat's instrument was used	
Initial (min)	120
Final (min)	280
Compressive strength for the cement paste cube	
3days (Mpa)	16
7days (Mpa)	25

**Table No.3 Sieve analysis of the fine aggregate**

Sieve size (mm)	Commulative passing%	% passing of the overall limit of ASTM C33-03 (30)
9.5	100	100-100
4.75	97	95-100
2.36	92	80-100
1.18	72	50-85
0.6	41	25-60
0.3	14	5-30
0.15	4	0-10

**Table No.4** Sieve analysis of the coarse aggregate

Aggregate Id	Sieve Size(mm)	Cumulative Passing%*	%Passing of the overall limit of ASTM C33-03{30}
G10	9.5	96	85-100
	4.75	25	10-30
	2.36	8	0-10
	1.18	0	0-5
G19	19	98	90-100
	4.2	51	20-55
	4.75	4	0-10
	2.36	0	0-5
G38	37.5	96	95-100
	11.2	45	35-70
	9.5	21	10-30
	4.75	3	0-5

**TABLE NO.5** Properties of fibre provided by the manufacturer

Fiber type	material	Length(i f)(mm)	Diamete r(df)(m m)	Aspect ratio(if/ df)	Tensile strength Mpa
Microsteel fibre	Steel	13	0.2	65	>2100
Hooked end	Steel	35	0.55	64	900-2200
Hooked end	Steel	60	0.75	80	900-2200
Macro synthetic	Copolymer / polypropene	19	0.34	56	570-660
Macro synthetic	Copolymer / polypropene	38	0.34	112	570-660
Micro synthetic	Copolymer / polypropene	54	0.34	168	570-660



a)



b)





c) d)



e) f)

**Table6.Mixconstituents**

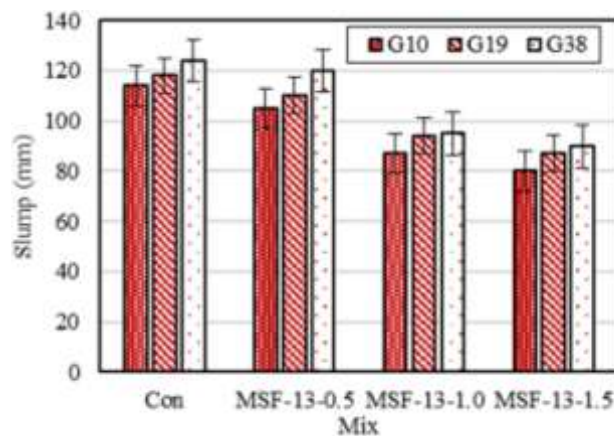
Constituents	Mix1	Mix2	Mix3
Cement (kg/m <sup>3</sup> )	450	450	450
Water (kg/m <sup>3</sup> )	203	203	203
Water/Cement ratio	0.45	0.45	0.45
Sand ( kg/m <sup>3</sup> )	723	723	723
Coarse Aggregate (Kg/m <sup>3</sup> )	1010	1010	1010
CAMZ (mm)	10	19	38
Fibre content (%) by volime	0/0.5/1.0/1.5	0/0.5/1.0/1.5	0/0.5/1.0/.5

### 4.3. Results and Discussions:

#### 4.3.1 Effect of Fiber Properties on Flowability

The effects of the fiber type, dosage, and CAMZ on the slump of fresh synthetic/steel FRC are depicted. It should be highlighted that regardless of the fiber type, the fiber inclusion into the concrete damagingly impacted the flowability of fresh FRC. For the micro steel fiber (MSF), maximum slump declines of 7.9%, 23.7%, and 29.8% occurred for MSF concrete with 0.5%, 1.0%, and 1.5% fiber dosages, respectively, compared with the control mix. Additionally, the hooked end steel fiber (HSF) added into the control concrete had higher negative effects on the slump results compared with the MSF mixtures, with the maximum slump decreases being around 10%, 28%, and 34% for HSF concrete with 0.5%, 1.0%, and 1.5% fiber dosages, respectively, f

or both fiber lengths (35 mm and 60 mm). Moreover, Figure, shows greater influences from the longest fiber lengths of the HSF and synthetic fiber (SYF) on the flowability compared with the short fibers. The reduction in the fresh concrete flowability with incorporated synthetic/steel fiber might be attributed to the scattering of the fiber in fresh mixture concrete, which established interfacial bonding between the fibers and the concrete matrix. In Figure, the results of the slump test reveal that the slump of fresh synthetic/steel fiber mix produced a decreasing tendency with the synthetic/steel fiber length's increase. Thus, the impact of the fiber-concrete matrix took the lead for the decrease of the slump, even though the number of fibers was lower in the same fiber dosage with the same volume of concrete.

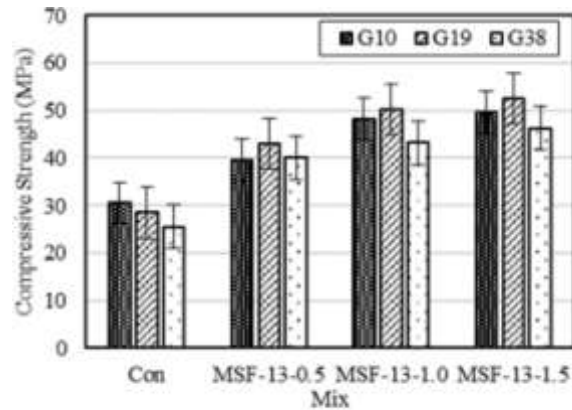


Furthermore, the slump of fresh synthetic/steel mixture along with control mixture enhanced as the CAMZ increased from 9.5 mm to 37.5 mm. It should be noted that the increase of the CAMZ may decrease the small particle content in the aggregate, which negatively influences the amount of concrete mixture covering the coarse aggregate, causing an increase of the slump.

#### 4.3.2 Effect of Fiber Properties on the Compressive Strength

The compressive strength of the control concrete was reduced by increasing the CAMZ, and the percentage decreases were 6.6% and 16.6% for the Con-G19 and Con-G38 mixtures, respectively, compared with the Con-G10 mixture. For specimens with incubating synthetic/steel fiber, the results display that the compressive

strength of the FRCs was improved by the incorporation of fibers, and the maximum percentage increases were around 62%, 85%, and 82% for the MSF-13-1.5-G10, MSF-13-1.5-G19, and MSF-13-1.5-G38 mixtures, respectively, compared with the Con mixtures. It should be highlighted that regardless of the fiber type and dosage, the fiber inclusion into the concrete significantly impacted the compressive strength of the FRC. This enhancement may be due to the fact that the fibers can bridge microcracks and macrocracks that form in the concrete matrix. It should be highlighted that the compressive strength was only marginally affected by the CAMZ compared with the fiber dosage and geometrical properties, such as the length and diameter, which had an insignificant influence on the compressive strength



(a)

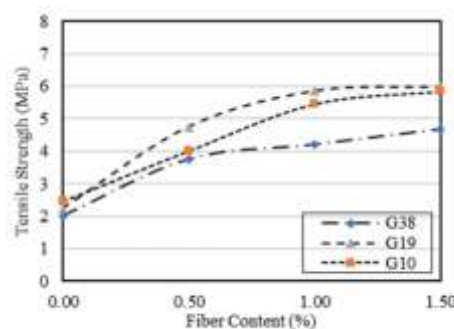
The greatest improvement of the compressive strength was provided by using the MSF with a 1.5% fiber dosage along with a 19mm CAMZ (G19). This may be due to the fact that the MSF had a higher fiber content among the other fiber types in the same fiber dosage.

#### 4.3.3 Effect of Fiber Properties on the Tensile Strength

The effects of the fiber type, dosage, and CAMZ on the splitting tensile strength are represented in Figure. The splitting tensile strength of the control concrete was reduced by increasing the CAMZ, and the percentage decreases were 8.6% and 18.0% for the Con-G19 and Con-G38 mixtures, respectively, compared with the Con-G10 mixture. On the other hand, the results show that adding fiber into the concrete significantly enhanced the tensile strength, with the greatest improvements of 165%, 193%, and 231% occurring for the SYF-54-1.5-G10, SYF-54-1.5-G19, and SYF-54-1.5-G38 mixtures, respectively, compared with the

control mixtures. This behavior is primarily recognized as the bond between the concrete matrix and the fiber. Generally, the improvement of the FRC behavior was much more significant in the tension behavior than in the compression behavior.

For the specimens with steel fiber, it can be seen that the splitting tensile strength of MSF provided the highest increase compared with the other concrete mixture with hooked end steel fiber types, while the splitting tensile strengths of the synthetic fiber specimens progressively enhanced with the increase of the length of the synthetic fiber. The splitting tensile strength improvement of the synthetic fiber specimens might be due to the synthetic fibers being pulled out after debonding between the fiber and concrete matrix rather than being broken. Thus, a longer embedment length of the synthetic fiber into the concrete matrix can provide larger pullout forces.



(a)

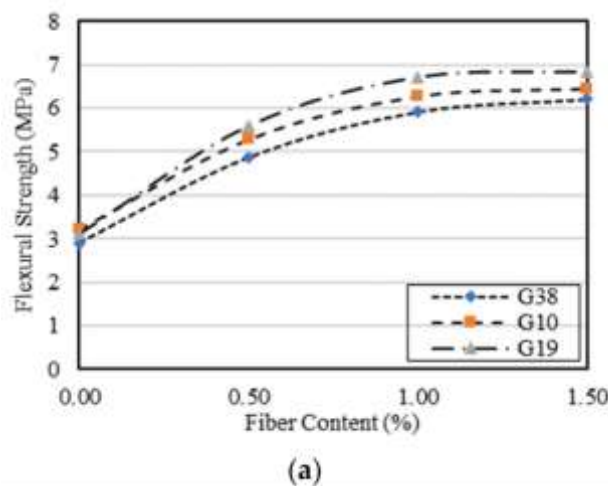
The tensile strengths of the FRC specimens improved with a CAMZ up to 19 mm and then declined with a CAMZ of 37.5 mm, as shown in Figure 5. With the rise of the CAMZ, the total surface area of the coarse aggregate reduced, which decreased the concrete matrix quantity around the aggregate. This may have impacted the fiber-embedded matrix, which led to providing weak pullout forces. Therefore, with the optimum CAMZ, the bonding strength at the interface between the concrete matrix and fiber could be enhanced, and thus the strengthening influence of the fiber on the concrete strength would be enhanced. Nevertheless, the large CAMZ might have a disadvantage in the distribution of fiber in concrete, reducing the strengthening impact of fiber on the concrete strength.

The change rules of the splitting tensile strength ratio of the FRC to the splitting tensile strength of the control concrete with various ratios ( $R_t$ ) of the fiber length to the CAMZ multiplied by the fiber dosage ( $R_t = (l_f / \text{CAMZ}) \cdot f_d$ ). In this figure, the splitting tensile strength ratio significantly rises with the ratio ( $R$ ) increase with good correlation. In addition, there is a significant relationship between the tensile strength ratio and ratio ( $R_t$ ) for all CAMZs. This is primarily accredited to the crack bridging effect formed by the fibers. The bridging effect developed by fibers crossing cracks was improved with the increase of the  $R_t$  factor, which enhanced the FRC mechanical performance. The results demonstrate that the fiber reinforcing effect on the tensile strength was substantial, with the ratio ( $R_t$ ) of the fiber length to the CAMZ multiplied by the fiber do-

sage ( $R_t = (l_f / \text{CAMZ})$ )

#### 4.3.4 Effect of Fiber Properties on the Flexural Strength

It should be highlighted that the CAMZ decreased the flexural strength by 2.8% and 9.7% for the Con-G19 and Con-G38 mixtures, respectively, compared with the Con-G10 mixture. For the effects of adding synthetic/steel fiber into the control concrete, the flexural strength significantly enhanced, and the maximum increments were 115%, 137%, and 153% for the SYF-38-1.5-G10, SYF-38-1.5-G19, and SYF-38-1.5-G38 specimens, respectively, compared with the Con specimens. This behavior of flexural strength improvement was attributed to the bridging fiber effect, which carried the load after the concrete matrix started cracking until the interfacial debonding between the fibers and the matrix (pulled out) or fiber rupture occurred. In general, the flexural strength of synthetic/steel FRC depicted a trend of improving as the fiber dosage increased. However, the amount of improvement depended on the fiber configuration, as shown in Figure. Besides that, the increase of the flexural strength with a CAMZ of 38 mm showed the greatest increase for the synthetic and hooked end steel fiber specimens, while the micro steel fiber specimens exhibited better performance with a CAMZ of 19 mm. Moreover, most of the hooked end fiber had pulled out from the concrete matrix while synthetic fibers were still involved in load transferring across the cracks, as shown in Figure. This action was mostly attributed to the configuration of each fiber type.





As can be noted, the synthetic fibers had a larger surface area than the hooked end fibers, causing a higher bond strength between the synthetic fiber and the concrete matrix. Additionally, the same explanations drawn for the differences in tensile strength with the rise of the fiber dosage, length, and CAMZ were primarily accountable for the differences in flexural strength.

Another approach would be to take the effect of the synthetic and steel fiber parameters (fiber length, diameter, and shape) along with the CAMZ on the compressive-to-flexural-strength ratio as the brittleness ratio. The brittleness ratio is the ratio between the compressive strength and the flexural strength listed in Table. For the synthetic and steel FRC specimens, the brittleness ratio and percentage changes are presented in Table. The low tensile capacity was causing early damage to the concrete, which corresponded to a high brittleness ratio of the concrete. As shown in Table, the Con specimens for all CAMZs exhibited the highest brittleness ratios among all synthetic and steel fiber specimens due to the low flexural strength and splitting tensile strength. This may be due to the inadequate hooked end fiber performance with a low fiber dosage and small CAMZ.

## V. CONCLUSION

In the present study, and analysis of the effects of steel and synthetic fiber parameters (fiber length, diameter, shape, and dosage) along with different CAMZs on mechanical properties, the following conclusions were drawn based on the study.

1. The slump test revealed that the slump of fresh synthetic/steel fiber mix produced a decreasing tendency with the synthetic/steel fiber length's increase. This decline in the fresh concrete flowability might have been due to the fiber length increase, which raised the overlap between fibers. Thus, the impact of the fiber-concrete matrix took the lead for the decrease of the mix slump, even though the number of fibers was lower in the same fiber dosage with the same volume of concrete.
2. The compressive strength was only marginally affected by the CAMZ compared with the fiber dosage and geometrical properties, such as the length and diameter, which had an insignificant influence on the compressive strength.
3. The splitting tensile strength of the FRC specimens improved with a CAMZ up to 19 mm and then declined with a CAMZ of 37.5 mm. With the rise of the CAMZ, the total surface area of the coarse aggregate reduced, which decreased the concrete matrix quantity covering the aggregate. Therefore, the large CAMZ might have had a disadvantage in the fiber distribution of fiber in the concrete that reduced the strengthening impact of the fiber on the concrete strength.
4. The synthetic fibers had a larger surface area than the hooked end fibers, causing a higher bond strength between the synthetic fiber and concrete matrix. In addition, the same explanations drawn for the differences in tensile strength with the rise of the fiber dosage, length, and CAMZ were primarily accountable for the differences in flexural strength.
5. The flexural strength ratio significantly rose with the ratio ( $R_f$ ) increase with good correlation. Additionally, there was a significant relationship between the flexural strength ratio and ratio ( $R_f$ ) for all CAMZs.
6. The mechanical properties of synthetic and steel FRC could be influenced by many factors, such as the fiber type, steel fiber geometry, fiber parameters, aspect ratio, and fiber dosage.
7. It should be highlighted that the regression analysis investigation was conducted for all compressive strengths, splitting tensile strengths, and flexural strengths of the synthetic and steel FRC with the consideration of these factors. These factors were a fiber length from 13 mm to 60 mm, CAMZ from 9.5 mm to 37.5 mm, and ratio of the fiber length to the CAMZ in the range of 0.35–5.68. Therefore, it can be noted that there were strong correlations from the regression analysis of the mechanical properties results of synthetic and steel FRC.

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