

# Industrial Waste Based Retaining Wall under Stress and Strain Load Analysis

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**ABSTRACT:** The amassing of mechanical waste has represents a major issue to the modern development and to human residence. Removal of mechanical waste is covering huge track of significant land. In this investigation an endeavor has been made to assess the properties of modern squanders like fly debris, red mud, smasher dust, impact heater slag to use as establishment bed and refill in Retaining structures. As the vast majority of the industrials squanders are dumped as stacks, considers have been completed for the footings inserted in inclining ground. Different explorations have been done in the seismic bearing limit of footings for even ground, yet the investigation for slanting ground is constrained. The impact of seismic powers on the above footings is likewise contemplated utilizing limited component technique. The slanted Retaining walls with modern squanders as inlay was broke down. As the variety in geotechnical properties of mechanical squanders are evident because of different reasons, ultimately, unwavering quality examination of establishments on modern waste utilizing convention limit balance technique is additionally contemplated, considering the inconstancy of the boundaries adding to the presentation of the framework.

**IndexTerms** – Retaining wall, Force, Displacement, Industrial Waste.

## I. INTRODUCTION

Since the first investigation of [1] and diagnostic investigation of [2], there have been a few exploratory, systematic and numerical investigations of the dynamic conduct of Retaining walls (RWs) because of offer a strategy for levelheaded displaying. The different procedures applied to examine dynamic earth weights can be distanced into three fundamental techniques, for example, investigative, numerical, and exploratory. While a few writing is done in the subject of seismically initiated parallel earth pressures. An

ongoing option in contrast to the Mononobe-Okabe (M-O) strategy for plastic soils was introduced by [3]. They offered a shut structure pressure pliancy answer for gravitational and seismic tremor incited earth pressures on RWs. Also, [4] and [5] as of late did isolate shake table tests utilizing axis offices, and both independently came about that the deliberate earth pressure during shaking was lower than the M-O strategy expectations. Nakamura [4] likewise featured that the inertial power was not generally transmitted to the wall and inlay at the same time. [6] Carried out rotator dynamic excitation tests with fixed-base cantilever walls supporting soaked, liquefiable, attachment less refills. In light of the outcomes, [6] found that overabundance pore pressure age expanded altogether to seismic sidelong earth pressure in the soaked inlay. They likewise call attention to that the greatest unique push was relative to the info base increasing speed. [7] planned the powerfully prompted horizontal earth pressure on the stem part of a solid cantilever earth RW with dry medium thick sand utilizing limited contrast code FLAC and established that at extremely low degrees of seismic action, the seismic earth pressures were in concurrence with M-O expectations; in any case, as increasing speeds expanded, seismic earth pressures were bigger than those determined by the M-O strategy. [8] Completed models of L-molded walls, pre-focused on tied down heap walls, and fortified soil walls, utilizing both straight and non-direct soil models. Utilizing those models, [8] introduced that including sensible impacts, for example, the wall adaptability, establishment soil deformability, material soil yielding and soil wall division and sliding will in general diminish the impacts of dynamic excitations on those walls. They likewise utilized a FE model to recreate a case history in which a RW performed well during a real seismic tremor. [9] Implemented an investigation to affirm the suspicions of Veletsos and Younan systematic arrangement and to proposed the range if its

pertinence. The numerical plans were introduced utilizing the business limited component technique (FEM). The adaptability of the numerical strategies, limited component and finitedifference, allowed the treatment of increasingly sensible circumstances that are not agreeable to explanatory arrangement including the heterogeneity of the held soil, and translational adaptability of the wall establishment. To explore the qualities of the parallel seismic soil pressure on building walls, [8] played out a progression of soilstructure-connection examinations utilizing SASSI. Utilizing the idea of a solitary level of-opportunity, proposed a disentangled technique to anticipate most extreme seismic soil pressures for building walls laying on firm establishment material. This proposed strategy brought about powerful earth pressure profiles similar to or bigger than the Wood [5] arrangement, with the most extreme earth pressure happening at the head of the wall.

## II. LITERATURE REVIEW

(Belal and George, 2000) specifically compelling in this investigation are: (1) the quickening reaction, (2) the wall dislodging, (3) the elastic worry in the support, and (4) the slippage at the dirt fortification interface. (Athmarajah and De Silva, 2019) To foresee the horizontal wall twisting for unearthings, it is essential to play out a numerical examination, in light of the fact that the avoidance of the fighter heap Retaining wall is basic by and by. In the current investigation, hypothetical examination and numerical investigation were completed to break down the strength of the warrior heap Retaining wall with the nearness of overcharge. (Laba, Kennedy and Seymour, 1983) Experimental outcomes were contrasted and the hypothetical pressure appropriation that exists in a semi-limitless flexible medium, and furthermore with structure system as of now being used for fortified earth Retaining walls under level extra charge strip stacking. Critical contrasts were found between the outcomes dependent on the plan technique now being used and those acquired from the model examination. Key (Lamerdonov et al., 2018) The Mountain Rivers during the flood time frames, regularly wash away waterfront fastenings and wreck streets because of side water disintegration. Imaginative advancements on fortifying of shoreline of the streams are advertised. (Xu et al., 2019) The plan of different systems in the current exploration prompted the best assessment of dynamic states of the Retaining wall. Under these conditions, an ACO was utilized for ideal plan. Impacts of boundaries shifted because of various wall conditions when dynamic burdens

were forced. (Yin et al., 2018) By recreating well known files to the SSD-based file framework and pushing the HDD-based file framework into the low-power mode under light remaining burden conditions, DuoFS can lessen significant vitality utilization, stay away from main considerations that hurt the capacity frameworks dependability, and concentrate SSDs great I/O execution. Trial results show that the DuoFS framework sets aside to 40% of vitality, accomplishes up to half better I/O execution while just sacrificing under 15% of the framework's unwavering quality. (Zhu et al., 2019) This paper presents the plan and advancement of a wellbeing observing and cautioning framework for profound establishment pits and contiguous structures. (Hu et al., 2019) presents a simplified approach for figuring aloof earth pressures for pit-in-pit unearthings. A trapezoidal-molded disappointment wedge is framed between two degrees of Retaining walls. Satyanarayana et al. [9] they can be perfect contenders for the non-inexhaustible, extravagant oil basically based manufactured filaments in composite substances, specifically inside the car venture and tallying building divisions. Andrzej et al. [10] have examined the effect of the sort of fortifying fiber, fiber and microvoid content material at the mechanical properties of composites. expanding the fiber content material convinces an expansion in the impact quality and shear modulus. Verma et al. [11] The goal in their view changed into to use the gifts offered by methods for sustainable hotspots for the improvement of composite materials dependent on bagasse filaments. They presume that hybridization with certain amounts of engineered strands makes these home grown texture composites progressively fitting for specialized applications along with car inside parts. Herrera-Franco et al. [12] They found that the resulting vitality and firmness of the composite relies upon the measure of silane stored at the fiber and the flexible modulus of the composite didn't improve with the fiber surface change. Rokbi et al. [13] The test results show that the twisting conduct of composites made from salt dealt with filaments are higher contrasted with the untreated fiber composite, For a fiber handling Alfa 10% NaOH in 24h, the flexural power and flexural modulus improved by 23 MPa to 57MPa and from 1.16 to a few.04 GPa. Oboh et al. [14] They reason that with regards to the morphosynthesis, the capability of replication of the luffa wipe loosens the potential outcomes of utilizing biodiversity in procuring new substances. Yoldas Seki et al. [15] They found that flexural power, elastic vitality, ductile lengthening and interlaminar shear power at harm estimations of the composite decreased by

method of 28%, 24%, forty five%, and 31%, separately, after water becoming more established. Lassad Ghali et al. [16] They detailed that the acetylation treatment propelled the mechanical homes of composites. The strategy diminished the hydrophilic conduct of the luffa strands, upgrading their bond to polyester network. Msahli et al. [17] They examined the utilizes of assorted fortification structures. Demir et al. [18] They build up that the Tensile power and more youthful's modulus duplicated with work of the coupling venders conveyed by means of a diminishing in water assimilation with cure because of the better attachment among the fiber and the grid. Xingwen Yin, Quandong Xiao. [19] Have explored that reestablishing power model can more readily mirror the hysteretic execution of the examples. It very well may be utilized as a source of perspective for flexible plastic seismic reaction investigation of DWPC shear wall structures. Wei Gao et al. [20] They find that yield voltage of the sensor is corresponding to the 33% intensity of wall shear pressure. In addition, contrasted and run of the mill Ni thermistor sensor, the affectability of SWCNTs warm sensor could be improved four to multiple times. [21] FE examination were poor down for load-avoidance conduct, break designs, extreme burdens; and the FE were in like manner differentiated and the test results. The assessment reveals that the model predicts the lead of shear essential glass fiber strengthened polymer (GFRP) fortified solid bars with reasonable degree of precision. [22] Samples taken were fruitless in flexure and delamination was the changing disappointment mode for all the reinforced samples. Burden conveying limit of fortified samples amplified from 27%-128% over the unstrengthen control test. It was likewise seen that the pliability and firmness of the examples expanded in line to the utilization of ideal cross breed overlays. [23] The F1-quantity attained in the help dataset was 63% which is still a long way from gigantic. Be that as it may, given the attributes of the confirmation information, these outcomes are rousing since 1) model isn't misrepresented by content from other interpersonal organizations and 2) approval dataset was controlled to a specific time span and specific watchwords (which can influence the introduction of the model). [24] Try a blunder strategy for finding comparative conduct to the investigational dimension of a blastencumbered fiber strengthened chunk was obtainable. The prerequisite of the difference in yield boundaries as bounce back is concentrated to the difference in the estimations of the information boundaries, for example, the quality of the solid and break vitality. [25] Relative survey

of the basic and non-auxiliary properties of two referenced materials dependent on the past investigations of the creators. The gave proof is extremely valuable to common planners to appropriately apply steel-fiber-strengthened cements (UHP-SFRC) and superior steel-fiber fortified cements (HP-SFRC). [26] By looking at drive introduction test results and recreation investigation results, the comparing circuit of fortified cement was surveyed. The two outcomes concur well, meaning that fortified cement can be given off a role as an earth anode. [27] The assessment show that mindful quality issues, including unsatisfactory elasticity, unacceptable yield quality, inadmissible weight and unsuitable arrangement, just as deficient entombment profundity of the two shafts during development are the primary driver of crack. [28] The guideline of the steel fiber strengthened cement (SFRC), are included steel filaments. Their amount influences hardness and consistency influences quality. SFRC is generally utilized as building material, yet nonappearance of administrative system for structuring, anticipating and rating of SFRC results at disunity in development practice. Which depends on concocting and verification of nondestructive testing (NDT) techniques for estimation picked characteristics of SFRC. [29] The hysteresis bends and skeleton bends acquired from the test were analyzed, and the connection between the dislodging malleability of fortified solid segments and three controlling elements was gotten.

As the vast majority of the industrials squanders are dumped as stacks, examines have been completed for the footings installed in slanting ground. Different exploration has been done in the seismic bearing limit of footings for even ground, yet the investigation for inclining ground is restricted. The impact of seismic powers on the above footings is additionally examined utilizing limited component technique. The slanted Retaining walls with modern squanders as inlay were dissected. As the variety in geotechnical properties of mechanical squanders are evident because of different reasons, finally, dependability investigation of establishments on modern waste utilizing convention limit balance strategy is additionally examined, considering the inconstancy of the boundaries adding to the presentation of the Retaining wall framework.

### III. METHOD OF ANALYSIS

#### 3.1 Finite Element Method

The limited component Method has been utilized in numerous fields of designing more than forty years. The limited component strategy is a

noteworthy part of soil mechanics to anticipate soil conduct by constitutive conditions. This permits architects to comprehend different kinds of geotechnical designing issues, particularly issues that are naturally unpredictable and can't be explained utilizing customary investigation without making improved suspicions. To reproduce the specific conduct of soil, different soil models have been created and to improve the investigation system different programming bundles have been created. The examination was on six-gestured triangles in the component. This 15 – noded triangle was grown subsequently expanding the quantity of hubs in the component. The use of 15-noded triangle is the least complex component for any examination in axisymmetric. At that point the specialists De Borst and Vermeer execute the 15-noded triangle in MATLAB in this manner taking care of the issue of cone penetrometer. The advancement of MATLAB continues with the issue to illuminate the dirt structure cooperation impacts. This prompted the examination on shaft component by Klaas Bakker under the oversight of Pieter Vermeer. The result of the trial utilizing shaft component was appropriate to adaptable Retaining wall and later application to the investigation of adaptable footings and pontoons. Pastry specialist's work planned the usage of 5-noded shaft component in MATLAB (Bakker et al (1990), Bakker et al (1991)). The 5-noded bar component is perfect to the 15-noded triangular components (has 5 nodes). Baker's work was novel for the development of crossover technique presenting the removal of level of – opportunity to the component conduct. The absence of level of opportunity has made answer for decrease the quantity of factors in this manner improved the component.

### 3.2 Seismic Condition

The impact of pseudo-static even seismic tremor body powers on the bearing limit of establishments on inclining ground has been surveyed utilizing Finite Element Method. Two disappointment systems were thought of, in light of the augmentation of the attributes starting from the earliest stage towards the balance base from either onside or the two sides. The greatness of  $N\gamma$  dependent on the both-sides disappointment component, for littler estimations of seismic tremor increasing speed coefficient ( $\alpha h$ ), has been seen as fundamentally littler than that got utilizing the single side system; nonetheless, within the sight of  $\alpha h$  the two sides instrument turns out to be kinematically prohibited in .Only the single-side component was found statically many cases for higher estimations of permissible for figuring the

bearing limit factors  $N_c$  and  $N_q$  on inclining ground. All the bearing limit factors diminish extensively with increment in  $\alpha h$  for different ground tendencies.

## IV. MATERIALS

The following samples were used and some experiments were conducted on this sample for comparison and knowing about the soil behaviors for different materials. The experiments which were conducted are briefly discussed. The samples were collected from different site and the results concluded form the experiment gives the property of soil for that site only.

### 4.1 Fly Ash

The fly-ash is a fairly divided residue which results from the combustion of ground or powdered bituminous coal or sub-bituminous coal like lignite and transported by the flue gases of boilers fired by pulverized coal or lignite. Fly ash is generally captured by electrostatic precipitators or other particle filtration equipments before the flue gases reach the chimneys of coal-fired power plants, and together with bottom ash removed from the bottom of the furnace is in this case jointly known as coal ash. Depending upon the source and makeup of the coal being burned, the components of fly ash vary considerably, but all fly ash includes substantial amounts of silicon dioxide ( $SiO_2$ ) (both amorphous and crystalline) and calcium oxide ( $CaO$ ), both being endemic ingredients in many coal-bearing strata. Fly-ash contains some un-burnt carbon. It is acidic in nature and its main constituents are silica, aluminum oxide and ferrous oxide.

### 4.2 Red Mud

Red mud is a solid waste product of the Bayer process, the principal industrial means of refining bauxite in order to provide alumina as raw material for the electrolysis of aluminium by the Hall–Héroult process. Red mud is composed of a mixture of solid and metallic oxide-bearing impurities, and presents one of the aluminium industry's most important disposal problems. The red colour is caused by the oxidised iron present, which can make up to 60% of the mass of the red mud. In addition to iron, the other dominant particles include silica, unleached residual aluminium, and titanium oxide. Red mud cannot be disposed of easily. In most countries where red mud is produced, it is pumped into holding ponds. Red mud presents a problem as it takes up land area and can neither be built on nor farmed, even when dry. Due to the Bayer process the mud is

highly basic with a pH ranging from 10 to 13. Several methods are used to lower the alkaline pH to an acceptable level to decrease the impact on the environment.

#### 4.3 Crusher Dust

When furnaces and quarrying procedures produce slag from treating different types of stone, manufacturers gather this slag together and grind it down into crusher dust. This dust is made of a variety of materials, but often contains a large amount of silicates and alumina-silicates. In appearance, crusher dust has a greyish or brownish tone with very fine aggregate particles, like soft sand. These particles, when looked at under a microscope, are rough cubes and individually have a rough surface texture. Crusher dust has many of the useful properties of the stone that it comes from. It is very heat resistant and contains no plastic chemicals that may be toxic to the surrounding environment over time. The chemical nature of crusher dust is very dependable and largely alkaline. So that it can be used in variety of material. It is also durable, strong, and can be easily compressed into tight spaces. Crusher dust is primarily used as filler and cement aggregate. Sometimes it can also be used as the replacement for fine aggregates in the concrete. When used in concrete, the crusher dust mixes in with larger aggregate to help form a specific texture. The dust is also used to make mortar and other similar materials.

#### 4.4 Slag

Slag is a partially vitreous by-product of smelting ore to separate the metal fraction from the unwanted fraction. It can usually be considered to be a mixture of metal oxides and silicon dioxide. However, slag can contain metal sulphides and metal atoms in the elemental form. While slag are generally used as a waste removal mechanism in metal smelting, they can also serve other purposes, such as assisting in the temperature control of the smelting; and also minimizing any re-oxidation of the final liquid metal product before the molten metal is removed from the furnace and used to make solid metal. Ferrous and non-ferrous smelting processes produce different slag. The smelting of copper and lead in non-ferrous smelting, for instance, is designed to remove the iron and silica that often occurs with those ores, and separates them as iron-silicate-based slag.

#### 4.4 Laboratory Investigation

Soil Compaction Soil compaction is defined as the method of mechanically increasing

the density of soil. In construction, this is a significant part of the building process. If performed improperly, settlement of the soil could occur and result in unnecessary maintenance costs or structure failure.

Almost all improperly, settlement of the soil could occur and result in unnecessary maintenance costs or structure failure. These different types of effort in field are found in the two principle types of compaction force: static and vibratory.

- 1) Static force is simply the deadweight of the machine, applying downward force on the soil surface, compressing the soil particles. The only way to change the effective compaction force is by adding or subtracting the weight of the machine. Static compaction is confined to upper soil layers and is limited to any appreciable depth. Kneading and pressure are two examples of static compaction.
- 2) Vibratory force uses a mechanism, usually engine-driven, to create a downward force in addition to the machine's static weight. The vibrating mechanism is usually a rotating eccentric weight or piston/spring combination (in rammers). The compactors deliver a rapid sequence of blows (impacts) to the surface, thereby affecting the top layers as well as deeper layers.

Factors affecting Compaction:

Various factors affecting compactions are:

- 1) Water content
- 2) Amount of compaction
- 3) Method of compaction
- 4) Type of soil
- 5) Addition of admixtures

#### 4.5 Standard Proctor Compaction Test

The proctor test was developed by R.R Proctor in the year 1933 for the construction of earth fill dams in the state of California. The Indian standard IS: 2720 (part VII) was followed in the present study.

##### 1) Modified Proctor Test:

The modified proctor test was developed to give a higher standard of compaction. In this test the soil is compacted in the standard proctor test mould but in 5 layers instead of as in standard Proctor test. The Indian standard IS: 2720 (part VIII) was followed in the present study.

##### 2) Compaction Using Table Vibrator:

The samples were compacted in a table vibrator to find out the maximum and minimum void ratio.

##### 3) Specific Gravity:

This test is done to determine the specific gravity of fine-grained soil by density bottle method as per IS: 2720 (Part III/Sec 1) – 1980.

Specific gravity is the ratio of the weight in air of a given volume of a material at a standard temperature to the weight in air of an equal volume of distilled water at the same stated temperature.

#### 4.6 Reporting of Results

The specific gravity  $G$  of the soil is  

$$G = (W_2 - W_1) / [(W_4 - W_1) - (W_3 - W_2)]$$
  
 The specific gravity should be calculated at a temperature of 27°C and reported to the nearest 0.01.

If the room temperature is different from 27°C, the following correction should be done:

$$G' = kG \quad (2.1)$$

Where,

$G'$  = Corrected specific gravity at 27°C

$k$  = [Relative density of water at room temperature] / Relative density of water at 27°C.

#### 4.7 Direct Shear Test

A direct shear test also known as shear box test is a laboratory or field test used by geotechnical engineers to measure the shear strength properties of soil or rock materials or of discontinuities in soil or rock masses. IS: 2720 (Part XIII) was followed to establish the shear strength properties of soil.

#### 4.8 Retaining Wall

Retaining walls are constructed to support the backfill and designed to resist the lateral pressure of soil which otherwise move downwards. The purpose of retaining wall is to stabilize slopes. Retaining walls are employed in many engineering projects such as hill side roads, approach roads, bridges spillway of dams or costal structures. The value of active earth pressure plays a major role in design criteria and it depends on soil parameters. In the present study, active earth pressure was calculated for industrial wastes using the package MATLAB.

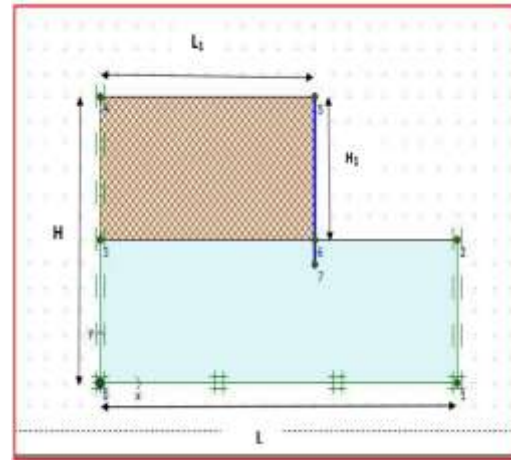


Figure 1.1: Schematic Diagram of Retaining Wall.

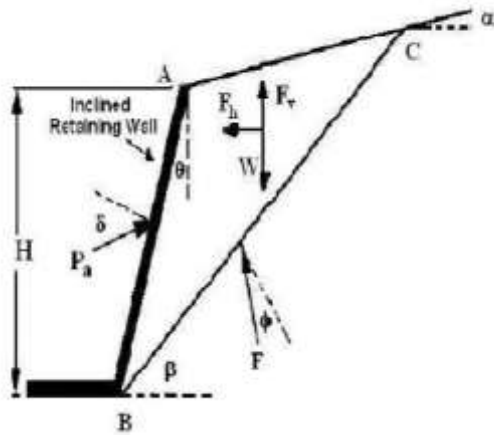
In our case study we have taken the dimensions as mentioned below:  $L=20$  m,  $L_1=12$  m  $H=12$  m and  $H_1=6$  m. The properties of the foundation soil are as follows:

Material model: Linear elastic  $\gamma_{unsat} = 18$  kN/m<sup>3</sup>,  $\gamma_{sat} = 20$  kN/m<sup>3</sup> Co-efficient of elasticity,  $E = 100000$  kN/m<sup>2</sup> The properties of plate retaining wall: Material type: Elastic  $EA = 3 \times 10^7$  kN/m,  $EI = 5 \times 10^7$  kN/m<sup>2</sup> /m, Poisson's ratio = 0.15 The aim is to find out the effective earth pressure on the retaining wall due to back fill. In the back fill we used the industrial wastes such as fly ash, slag, crusher dust and red mud.

## V. RESULTS AND DISCUSSION

### 5.1 Inclined Retaining Wall

The value of active earth pressure has direct relation to the angle of wall. It means by reduction of inclination angle from vertical state the value of active earth pressure will decrease. However only a few analytical solutions has been reported in design codes or published researches for calculating the active earth pressure which is usually smaller in inclined walls than vertical walls.

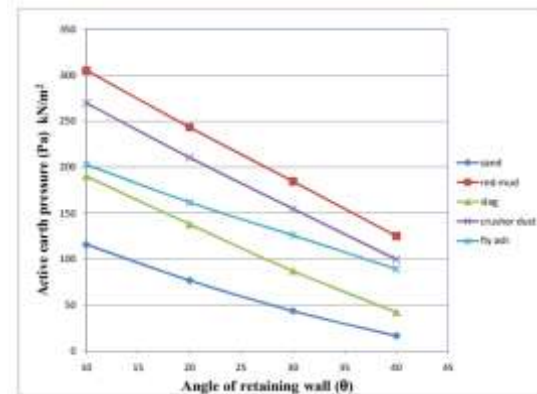


**Figure 1.2:** Schematic Diagram of Inclined Retaining Wall.

Ghanbari and Ahmadabadi (2009) have proposed several formulae to calculate the active earth pressure by considering limit equilibrium method. Necessary parameters are extracted assuming the pseudo static seismic coefficient to be valid in earthquake conditions.

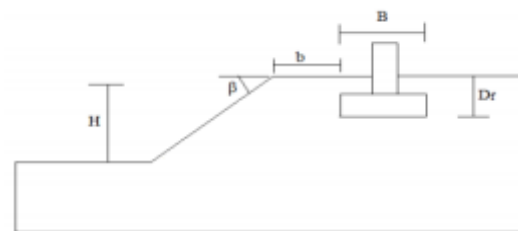
Using analytical relations based on equilibrium of forces and moments in a failure wedge, characteristics of active earth pressure in static and pseudo-static conditions for inclined walls is calculated using 'C' coding. In our work we have developed a 'C' program to calculate the active earth pressure.

Figure 1.3 shows the variation of active earth pressure on the retaining wall for different angle of inclination of the wall for different materials. It can be seen that earth pressure reduces with increase in angle of retaining wall. It was also observed that maximum active earth pressure was observed for red mud, followed by crusher dust, fly ash, slag and sand. High earth pressure value. Similarly the fly ash  $\phi$  of red mud pressure is due to its high density value and comparative low has considerably less earth pressure value due to its low density value.



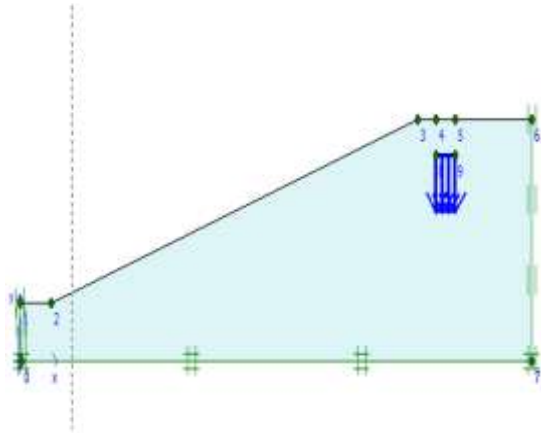
**Figure 1.3:** Active earth pressure against angle of inclination of wall for different materials.

The objective of this study is to determine the ultimate bearing capacity of a shallow continuous footing with width B in the presence of horizontal earthquake acceleration  $ah$  g (g is the acceleration due to gravity). The footing is placed horizontally on an inclined ground surface having an inclination  $\beta$  with the horizontal. It is assumed that the ground surface is loaded with a layer of soil overburden having equal vertical thickness, d, on either side of the footing. Figure 1.4 shows the geometry of the inclined ground with footing.

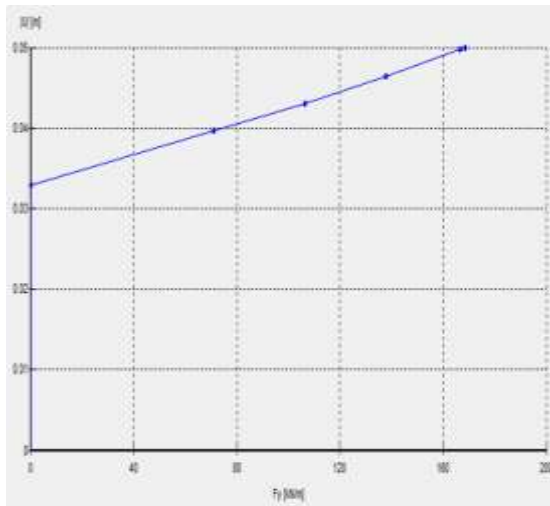


**Figure 1.4:** Geometry of inclined ground with embedded footing.

Data Given: Width of Footing  $B = 2$  m  
 Depth of Footing  $D_f = 2$  m  
 Height of the slope  $H = 6.2$  m  
 Angle of inclination  $\beta$  (varies for different cases)  
 Distance from top of slope to foundation  $b = 1.2$  m  
 Unit weight of soil  $= 17.5$  KN/m<sup>3</sup>  
 $\phi = 30^\circ$   
 Angle of Friction Cohesion  $c = 50$  KN/m<sup>2</sup>  
 STATIC LOADING )  
 CASE 1: ( $\beta = 15^\circ$   
 The Figure below shows the geometry of the sloping ground with embedded footing for an angle  $\beta$  of inclination of 15



**Figure 1.5:** Geometry of embedded footing for angle of inclination of 15.

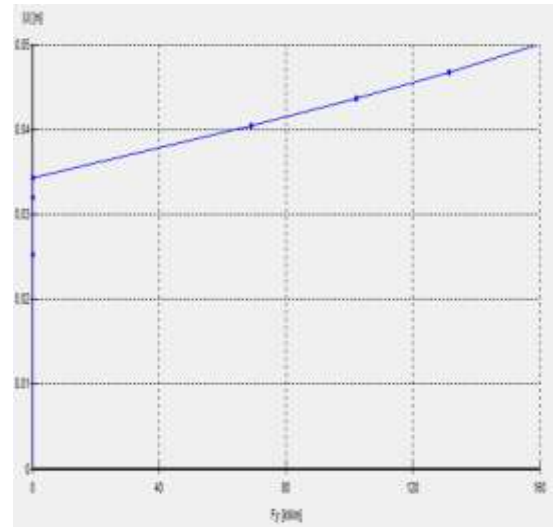


**Figure 1.6:** Force Vs Displacement graph for Case-1 (static load).

Ultimate load bearing capacity for the above footing is found to be 168.68 kN/m<sup>2</sup>

**SEISMIC LOAD:**

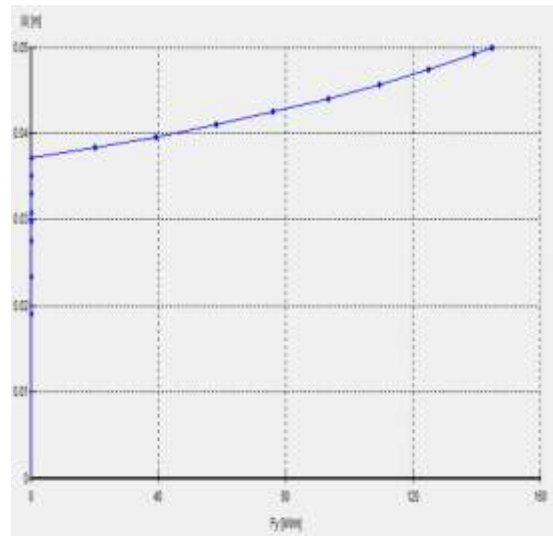
°CASE 1: ( $\beta = 15$  Horizontal acceleration = 0.1g)



**Figure 1.7:** Force Vs Displacement graph for Case-1 (seismic load)

Ultimate load bearing capacity for the above footing is found to be 157.88 kN/m<sup>2</sup>.

°CASE 2: ( $\beta = 15$  Horizontal acceleration = 0.2g)



**Figure 1.8:** Force Vs Displacement graph for Case-2 (seismic load)

Ultimate load bearing capacity for the above footing is found to be 154.65 kN/m<sup>2</sup>.

**VI. CONCLUSION**

Fast industrialization has brought about collection of tremendous amounts of modern waste. Removal of mechanical waste is covering immense track of significant land and furthermore dirtying condition. Cure lies in powerful use of these losses in enormous amounts. In this examination an endeavour has been made to assess the properties of mechanical squanders like fly debris, red mud,



smasher dust, impact heater slag to use as establishment bed and inlay in Retaining structures. An endeavour likewise has been made to utilize dependability examination for establishment on modern waste dependent on the properties of the losses according to research facility examinations. In light of the research center examination and limited component/limit balance investigation made thereof following ends can be made. 1 The modern squanders are seen as potential geotechnical designing materials. 2 The bearing limit found to diminish with increment in incline edge and furthermore diminishes with increment in seismic powers. 3 For the slanted Retaining wall, most extreme dynamic earth pressure was watched for red mud, trailed by smasher dust, fly debris, slag and sand. High earth pressure value of red mud might be because of its high thickness worth and relative low worth. So also the fly debris has significantly less earth pressure an incentive because of its low thickness esteem.

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